Technical Note

Enso Energy

June 2023

Southlands Solar Farm FRA Addendum

Project	Southlands Solar Farm FRA Updates			
Project Number	P23_174			
Title	Southlands Solar Farm FRA Addendum			
Description	This technical note aims to address and overcome the objections to the FRA produced for this proposed solar farm development.			
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Date	23 rd June 2023			
Version	1.0			



1 Introduction

1.1 Background

Wallingford HydroSolutions Ltd (WHS) has been commissioned by Enso Green Holdings J Ltd. ("the Applicant") to prepare an addendum to the previously prepared Flood Risk Assessment (FRA) and Outline Drainage Strategy (Document reference: WHS1973 Southlands Solar Farm FRA & Outline Drainage Strategy v2.0, 18th October 2022). The proposed site is solar farm located on land south of Runwell Road (A132), Runwell near Wickford, Essex (NGR: 576665, 194589).

This addendum aims to address and overcome the objections received from the Environment Agency (23/00285/FUL 12th May 2023).

1.2 Scope

To overcome the objection the following points will be addressed:

- Flood level data from the EA's detailed hydraulic model should be used to inform flood risk and confirm PV modules stowage height of 2m will be above modelled flood levels. Climate change uplift factors used in the hydraulic model for the key design events 1.0% and 0.1% AEPs (Annual Exceedance Probability) require updating.
- The Sequential Approach in the siting of the development should be applied. If development is in Flood Zones 2 or 3 the Exception Test must also be applied. It should be demonstrated that infrastructure can be provided above the flood level and remain operational.
- A topographic survey should be submitted.

1.3 Sources of Information

- Client supplied topographic survey¹
- 2017 River Crouch 1D/2D hydraulic model²
- Client supplied Alternative Sites Assessment³
- Environment Agency published climate change factors⁴

⁴ Peak river flow allowances available from: https://www.gov.uk/guidance/flood-risk-assessments-climate-change-allowances



 $^{^{1}}$ Topographic Survey Rayleigh Site 3 (drawing no. TX1402_RAY 3_T_01 to T_06). Texo DSI, August 2022. Available as Appendix A.

² River Crouch Fluvial Flood Model. Ch2m prepared for Environment Agency, May 2017.

³ Planning Statement, Appendix 2- Alternative Sites Assessment. Enso Energy, March 2023.

2 Detailed Hydraulic Model Flood Data

2.1 Available detailed model

A data request was submitted to the EA and a hydraulic model for the River Crouch⁵ was supplied which runs along the southern boundary of the site. This model informs Flood Zones 2 and 3 associated with the river. No detailed model data was held for the tributary that runs through the site. The 2017 model applies Climate Change (CC) factors to the 1.0% and 0.1% AEP events, but these have since been updated in 2022. Therefore, the model has been revised and rerun to determine the flood depths with the latest CC uplift factors. A review of the model inputs and results provided shows that Flood Zone 3 is based on the 'Baseline' (with defences) 1.0% AEP model run and Flood Zone 2 is based on the 'Undefended' 0.1% AEP model run (the worst-case scenario run).

2.2 Latest climate change uplift factors

For essential development within Flood Zone 3 the higher central allowance should be applied to peak river flows⁶. In the Anglian River Basin District and Combined Essex Management Catchment, the 2080s higher central allowance is 38%. Therefore, this CC uplift factor needs to be applied to the 1.0% and 0.1% AEP events.

2.3 Model rerun

CC is applied within the River Crouch model by applying the uplift to the hydrograph scaling factor in all of the ReFH boundaries (excluding boundary CR13a which is fixed). With no climate change uplift this value is set to 0.50. Therefore, for a 38% uplift this value has been increase to 0.69 in the updated CC runs. The model was rerun in Flood Modeller version 6.0 and TUFLOW version 2020-10-AE for the 1.0% AEP + 38% CC and 0.1% AEP + 38% CC flood scenarios. In addition to the above modification made to the event files the following changes were made for the model simulations to run through fully:

Table 1 - Model changes

Model Run	Modification	Impact
All	File paths updated to native drive from which model was rerun.	None
Baseline (defended) 1.0% AEP + CC	A Z flag was applied to the SX link at E:568374, N:190906 to prevent an error message (lowest ZC elevation along SX link above 1D node bed) causing the model to end during setup. This was within the Laindon portion of the model approximately 10km upstream of the site.	Negligible
Undefended 0.1% AEP + CC	A Z flag was applied to the SX link at E:574279, N:191470 to prevent an error message (lowest ZC elevation along SX link above 1D node bed) causing the model to end during setup. This was within the Wickford portion of the model approximately 4km upstream of the site.	Negligible
	An error was flagged which involved a 2D connection to a 1D node that didn't exist. The 1D node in question was an out of bank downstream boundary approximately 10km upstream of the site. The boundary is present in the baseline model so was copied and added to the undefended model to resolve the error. The provided simulation files indicate a slightly newer version of the 1D model was used but this was not provided within the model package hence the update.	Negligible

The resulting maximum flood depth plots are shown below with the site layout and boundary for context.

⁶ Peak river flow allowances available from: https://www.gov.uk/guidance/flood-risk-assessments-climate-change-allowances



www.hydrosolutions.co.uk

⁵ River Crouch Fluvial Flood Model. Ch2m prepared for Environment Agency, May 2017.

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To verify the modifications made to the model are negligible, the 0.1% AEP undefended scenario with no climate change was rerun. As this was used to inform Flood Zone 2 the flood extent should show a good match, see Figure 1. The flood extent overlaps with a very minimal difference in border thus demonstrating that the changes made have had a negligible impact at the location of interest.

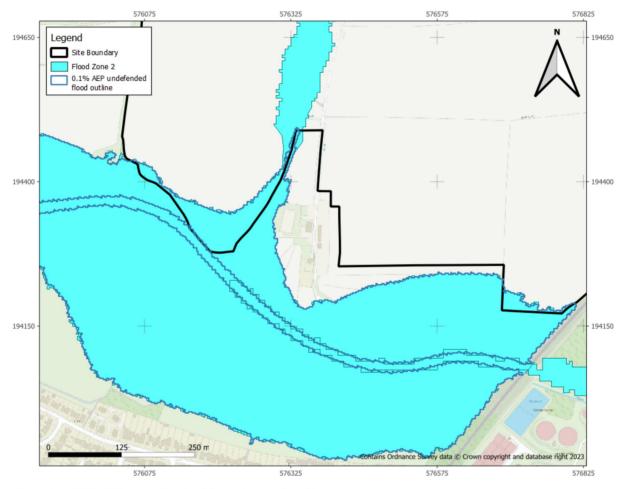


Figure 1 - 0.1% AEP rerun flood outline vs Flood Zone 2



2.4 100yr plus climate change flood depths

Figure 2 below shows the maximum flood depths associated with the 1.0% AEP flood event including a 38% climate change uplift factor.

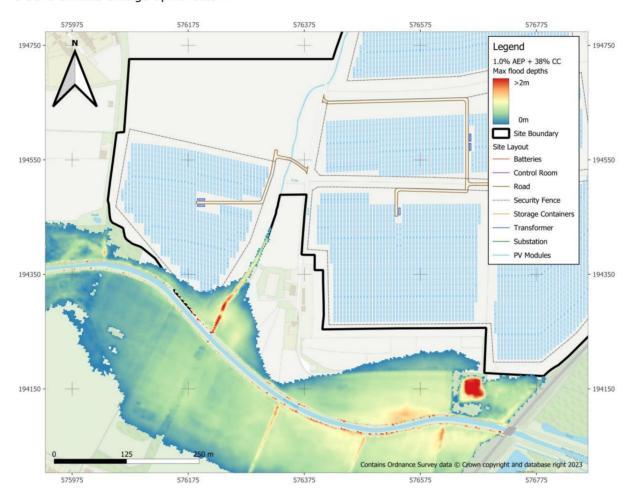


Figure 2 - 1.0% AEP + 38% climate change max flood depths (defended)

The maximum flood depth within the vicinity of the PV modules on the west side portion of the site is 0.15m. No PV modules are flooded on the east side. Therefore, for this design event the panel stowage height of 2.0m is sufficiently high enough to be above the flood level and provides a freeboard of 1.85m.



2.5 1000yr plus climate change flood depths

Figure 3 below shows the maximum flood depths associated with the 0.1% AEP flood event including a 38% climate change uplift factor.

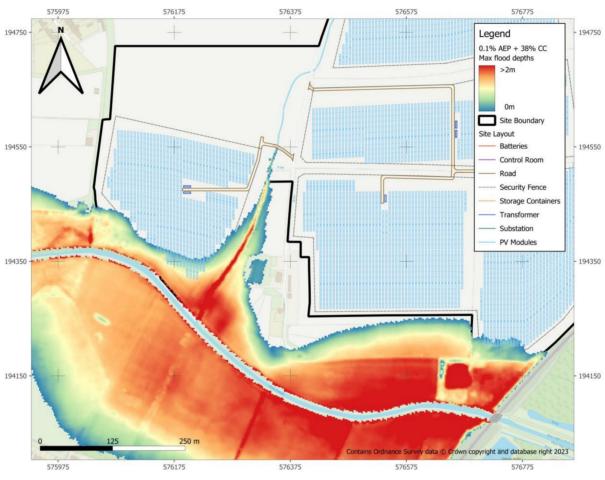


Figure 3 - 0.1% AEP + 38% climate change max flood depths (defended)

The maximum flood depth within the vicinity of the PV modules on the west portion of the site is 1.35m. In the east portion of the site the maximum flood depth within the vicinity of the PV modules is 0.60m. Therefore, for this design event the panel stowage height of 2.0m is sufficiently high enough to be above the flood level and provides a freeboard of at least 0.65m.

A 0.1% AEP +38% CC undefended scenario was also run as a sensitivity test to understand the impact of removing the upstream defences on the flood levels here. In this scenario the maximum flood depth in the vicinity of the PV modules was 1.50m. This is still below the stowage height but reduces the freeboard slightly to 0.50m.

In either scenario (100yr+cc or 1000yr+cc), the solar farm would remain safe and operational, and not lead to any negative impact outwith the site.



3 PV Panel Elevations

The proposed PV panels will have the ability to rotate around their centre to maximise their efficiency throughout the day. This has the advantage of being able to be stored horizontally as required, providing 2m of clearance above local ground levels, Figure 4.

During extreme flood events the PV panels will be stored horizontally, thus providing the freeboard identified in Section 2.4 and 2.5.

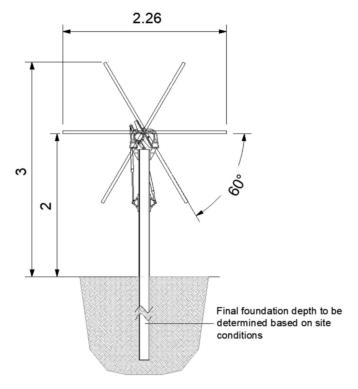


Figure 4 - Elevation View of proposed PV Panels



4 Sequential and Exception Tests

4.1 Sequential Test

The site is located in Flood Zone 2 and 3 and therefore the sequential test needs to be carried out which justifies its location. An Alternative Sites Assessment⁷ (Appendix 2 of the Planning Statement) was prepared by the client which reviewed multiple other sites based on a number of parameters on a scoring system. This included factors such as land availability, size, topography, ecology and flood risk. Ultimately the assessment concluded there was not a suitable available alternative site.

Upon development of the site layout within the boundary the Flood Zones were considered and the layout sequentially developed accordingly. All critical infrastructure at ground level including batteries, the control room, storage containers, transformers and the substation were located in Flood Zone 1 only. Due to the large number of PV modules and the space requirements some were located in Flood Zone 3. The panels can return to a stowage height of 2m; this is above the very worst case flood level and the equipment will not be damaged. Sufficient protection will also be provided to the connecting cables, which are waterproof.

The sequential approach has therefore been taken on a site scale as well as the development layout.

4.2 Exception Test

As the site is 'essential infrastructure' located in Flood Zone 3 the exception test must be completed. This test must demonstrate the following:

- The sustainability benefits of the development to the community outweigh the flood risk.
- The development will be safe for its lifetime taking into account the vulnerability of its users and that it won't increase flood risk elsewhere.

The proposed solar farm will provide an additional, urgently required, source of renewable energy and storage capacity to support the UK's goal to decarbonise all sectors and meet the net zero target by 2050. Flood risk to the solar farm is relatively small due to the sequential placement of equipment and the design height of solar panels on the site meaning the solar farm is sufficiently protected. The sustainability benefits therefore significantly outweigh the minor flood risk, which has been mitigated through design.

Key infrastructure has been placed outside of the Flood Zones with the exclusions of a small number of PV modules and an access track. The PV modules will return to a stowage height of 2m which is above the 0.1% AEP scenario flood level including the latest climate change uplift factor, as demonstrated in section 2. The panels will therefore not negatively impact flood mechanisms here by taking flood water storage space. The metal framework upon which the panels are placed will not impeed the flow of water. The access track will be permeable and at grade and therefore will not impede or impact flooding here. During an extreme flood event, users of the site (which will be infrequent) will not access the southwestern portion of the site and will evacuate to the east side of the site. The placement of the infrastructure and panel raising will therefore ensure there is no increase in flood risk elsewhere.

⁷ Planning Statement, Appendix 2- Alternative Sites Assessment. Enso Energy, March 2023.



5 Topographic Survey

A topographic survey⁸ was commissioned by the client as part of the planning application, this is available as Appendix A of this technical note. This has been compared with the 1m LiDAR data used to review topography in the FRA to confirm a good match. LiDAR typically has an accuracy of ± 150 mm. Several spot checks were conducted across the site with particular focus on the southwestern corner in the flood zones, see Figure 5. All comparisons were favourable, see Table 2, and showed a difference no greater than ± 100 mm. Therefore, the topographic survey and LiDAR are considered to be suitably accurate for use.

Table 2 - Topography Sample Points

ID	Easting	Northing	Topo Survey (m AOD)	LiDAR (m AOD)	Difference (m)
1	576170	194320	5.04	5.11	0.07
2	576210	194290	4.78	4.86	0.08
3	576240	194330	5.32	5.32	0.00
4	576240	194610	11.78	11.76	-0.02
5	576560	194880	17.97	18.07	0.10
6	576920	194990	22.68	22.69	0.01
7	576730	194670	21.42	21.47	0.05
8	577090	194620	14.45	14.4	-0.05
9	576780	194420	13.00	13.04	0.04
10	576570	194280	7.97	8.04	0.07

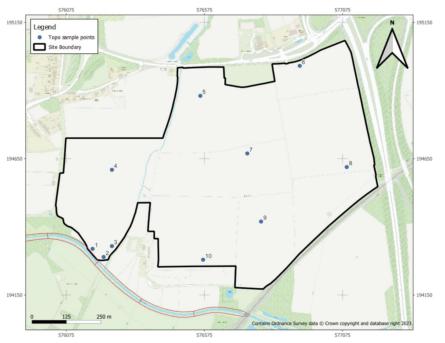


Figure 5 - Sample Point Locations

⁸ Topographic Survey Rayleigh Site 3 (drawing no. TX1402_RAY 3_T_01 to T_06). Texo DSI, August 2022.



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Appendix 1 - Topographic Survey



